

# Design Sensitivity Analysis and Optimization of Nonlinear Transient Dynamics

8th AIAA/USAF/NASA/ISSOMO Symposium on  
Multidisciplinary Analysis and Optimization

*Nam Ho Kim* and *Kyung Kook Choi*

Center for Computer-Aided Design  
College of Engineering  
The University of Iowa

# Contents

- **Introduction**
- **Structural Dynamics**
  - Multiplicative Elastoplasticity
  - Linearization and Tangent Stiffness
  - Implicit Time Integration
- **Shape Design Sensitivity Analysis**
  - Finite Deformation Elastoplasticity
  - Time Integration of Design Sensitivity Equation
  - Update Path-Dependent Variables
- **Numerical Examples**
  - Bumper Impact Problems
  - Pressurized Sheet Metal Stamping Problems

# Introduction

- **Shape Design Sensitivity Analysis of Nonlinear Structure**
  - Finite Deformation Elastoplasticity Using Multiplicative Decomposition of the Deformation Gradient
  - Classical Return Mapping Algorithm Is Preserved Using Principal Kirchhoff Stress and Logarithmic Strain
  - Design Sensitivity Equation Is Obtained at the Initial Domain and Then Transformed into the Current Domain to Recover the Updated Lagrangian Form
  - Path-Dependency Comes from the Intermediate Configuration and Plastic Evolution Variables
  - Exact Tangent Operator Yields Iteration-Free DSA

# Introduction *cont.*

## ➤ Design Sensitivity Analysis of Structural Dynamics

- Newmark Family Implicit Time Integration Is Used to Solve 2nd-Order Design Sensitivity Differential Equation with Homogeneous Initial Conditions
- Design Sensitivity Equation Solves for the Material Derivative of Acceleration at Converged Time Steps

Time Integration



Material Derivative of Displacement

- Sensitivity Equation Is More Efficient for the Implicit Time Integration Method Than the Explicit Method Compared to the Cost of Response Analysis

# Multiplicative Elastoplasticity

➤ Kinematics (Deformation Gradient)

$$\mathbf{F}(\mathbf{X}) = \mathbf{F}^e(\mathbf{X})\mathbf{F}^p(\mathbf{X})$$

➤ Principal Logarithmic Strain

$$\mathbf{b}^e = \mathbf{F}^e \mathbf{F}^{eT} = \sum_{i=1}^3 v_i^2 \mathbf{m}^i \quad \mathbf{e} = \begin{bmatrix} e_1 \\ e_2 \\ e_3 \end{bmatrix} = \begin{bmatrix} \log(v_1) \\ \log(v_2) \\ \log(v_3) \end{bmatrix}$$

➤ Kirchhoff Stress

$$\boldsymbol{\tau} = \sum_{i=1}^3 \tau_i^p \mathbf{m}^i \quad \mathbf{m}^i = \mathbf{n}^i \otimes \mathbf{n}^i$$

$\mathbf{n}^i$ : Principal Vector

$\tau_i^p$ : Principal Stress

Isotropic Material Assumption

# Constitutive Relation

## ➤ Trial Elastic Principal Stress

$$(\boldsymbol{\tau}^p)^{tr} = \mathbf{c}^e \mathbf{e}^{tr} \quad \mathbf{c}^e = (\lambda + \frac{2}{3}\mu)\mathbf{1} \otimes \mathbf{1} + 2\mu\mathbf{I}_{dev}$$

All incremental deformation is assumed to be elastic

## ➤ Yield Function

$$\begin{aligned} f(\boldsymbol{\eta}, e^p) &= \|\boldsymbol{\eta}\| - \sqrt{\frac{2}{3}}\kappa(e^p) & \boldsymbol{\eta} &= dev(\boldsymbol{\tau}^p) - \boldsymbol{\alpha} \\ &= \|\boldsymbol{\eta}^{tr}\| - (2\mu + H_\alpha(e^p))\gamma - \sqrt{\frac{2}{3}}\kappa(e^p) = 0 \end{aligned}$$

## ➤ Return Mapping Algorithm

$$\boldsymbol{\tau}_{n+1}^p = (\boldsymbol{\tau}^p)^{tr} - 2\mu\gamma\mathbf{N}$$

$$\boldsymbol{\alpha}_{n+1} = \boldsymbol{\alpha}_n + \gamma H_\alpha(e^p)\mathbf{N}$$

$$e_{n+1}^p = e_n^p + \sqrt{\frac{2}{3}}\gamma$$

# Linearization and Tangent Stiffness

## ➤ Tangent Stiffness Tensor

$$\mathbf{c} = \sum_{i=1}^3 \sum_{j=1}^3 c_{ij}^{\text{alg}} \mathbf{m}^i \otimes \mathbf{m}^j + 2 \sum_{i=1}^3 \tau_i^p \mathbf{c}_{\text{trial}}^i$$

$$\mathbf{c}^{\text{alg}} = \frac{\partial \boldsymbol{\tau}^p}{\partial \mathbf{e}^{\text{tr}}} = \mathbf{c}^e - 4\mu^2 AN \otimes \mathbf{N} - \frac{4\mu^2 \gamma}{\|\boldsymbol{\eta}^{\text{tr}}\|} [\mathbf{I}_{\text{dev}} - \mathbf{N} \otimes \mathbf{N}]$$

$\mathbf{c}^{\text{alg}}$  is the same as classical elastoplasticity  
in principal stress/strain space

## ➤ Linearization

$$a(\mathbf{z}, \bar{\mathbf{z}}) = \int_{\Omega} \tau_{ij} \varepsilon_{ij}(\bar{\mathbf{z}}) d\Omega \quad \text{Structural Energy Form}$$

$$a^*(\mathbf{z}; \Delta \mathbf{z}, \bar{\mathbf{z}}) \equiv \int_{\Omega} \left( \varepsilon_{ij}(\bar{\mathbf{z}}) c_{ijkl} \varepsilon_{kl}(\Delta \mathbf{z}) + \tau_{ij} \eta_{ij}(\Delta \mathbf{z}, \bar{\mathbf{z}}) \right) d\Omega$$

Updated Lagrangian Formulation

# Intermediate Configuration

## ➤ Update Intermediate Configuration

$$\mathbf{F}_n^{e^{tr}} = \mathbf{F}_n \mathbf{F}_{n-1}^{p^{-1}} = \sum_{i=1}^3 \exp(e_i^{tr}) \mathbf{m}^i$$

$$\mathbf{f}^p = \sum_{i=1}^3 \exp(-\gamma N_i) \mathbf{m}^i$$

Incremental Plastic  
Deformation Gradient

$$\mathbf{F}_n^e = \mathbf{f}^p \mathbf{F}_n^{e^{tr}}$$

$$\mathbf{F}_n^p = \mathbf{F}_n^{e^{-1}} \mathbf{F}_n$$

It is assumed that the incremental plastic spin is arbitrary

➡ Continuous intermediate configuration



# Variational Formulation of Structural Dynamics

## ➤ Weak Form

$$d(\mathbf{z}_{,tt}, \bar{\mathbf{z}}) + a(\mathbf{z}, \bar{\mathbf{z}}) = \ell(\bar{\mathbf{z}}), \quad \forall \bar{\mathbf{z}} \in Z$$

$$d(\mathbf{z}_{,tt}, \bar{\mathbf{z}}) = \int_{\Omega} \rho \bar{\mathbf{z}}^T \mathbf{z}_{,tt} d\Omega \quad \text{Kinetic Energy Form}$$

$$\ell(\bar{\mathbf{z}}) = \int_{\Omega} \bar{\mathbf{z}}^T \mathbf{f}^b d\Omega + \int_{\Gamma^h} \bar{\mathbf{z}}^T \mathbf{f}^h d\Gamma \quad \text{Load Linear Form}$$

- Variational Equation in Structural Domain
- 2nd-Order Differential Equation in Time Domain

## ➤ Initial Conditions

$$\mathbf{z}(\mathbf{x}, 0) = \mathbf{z}^0(\mathbf{x}) \quad \mathbf{x} \in \Omega$$

$$\mathbf{z}_{,t}(\mathbf{x}, 0) = \mathbf{z}_{,t}^0(\mathbf{x}) \quad \mathbf{x} \in \Omega$$

# Implicit Time Integration

## ➤ Newmark Method

### – Predictor

$${}^n \mathbf{z}_{,t}^{pr} = {}^{n-1} \mathbf{z}_{,t} + (1 - \gamma) \Delta t {}^{n-1} \mathbf{z}_{,tt}$$

$${}^n \mathbf{z}^{pr} = {}^{n-1} \mathbf{z} + \Delta t {}^{n-1} \mathbf{z}_{,t} + \left(\frac{1}{2} - \beta\right) \Delta t^2 {}^{n-1} \mathbf{z}_{,tt}$$

### – Corrector

$${}^n \mathbf{z}_{,t} = {}^n \mathbf{z}_{,t}^{pr} + \gamma \Delta t {}^n \mathbf{z}_{,tt}$$

$${}^n \mathbf{z} = {}^n \mathbf{z}^{pr} + \beta \Delta t^2 {}^n \mathbf{z}_{,tt}$$

$\beta, \gamma$ : Newmark Parameters

# Implicit Time Integration *cont.*

- Linearization [  $a_{\Omega}(\mathbf{z}, \bar{\mathbf{z}})$  Is Nonlinear w.r.t.  $\mathbf{z}$  ]

$n$  : Time  $t_n$

$k$  : Iteration Counter

$$\begin{aligned} d(\Delta \mathbf{z}_{,tt}^{k+1}, \bar{\mathbf{z}}) + a^*({}^n \mathbf{z}^k; \Delta \mathbf{z}^{k+1}, \bar{\mathbf{z}}) \\ = \ell(\bar{\mathbf{z}}) - a({}^n \mathbf{z}^k, \bar{\mathbf{z}}) - d({}^n \mathbf{z}_{,tt}^k, \bar{\mathbf{z}}), \quad \forall \bar{\mathbf{z}} \in Z \end{aligned}$$

- Acceleration Form

$$\Delta \mathbf{z}^{k+1} = \beta \Delta t^2 \Delta \mathbf{z}_{,tt}^{k+1}$$

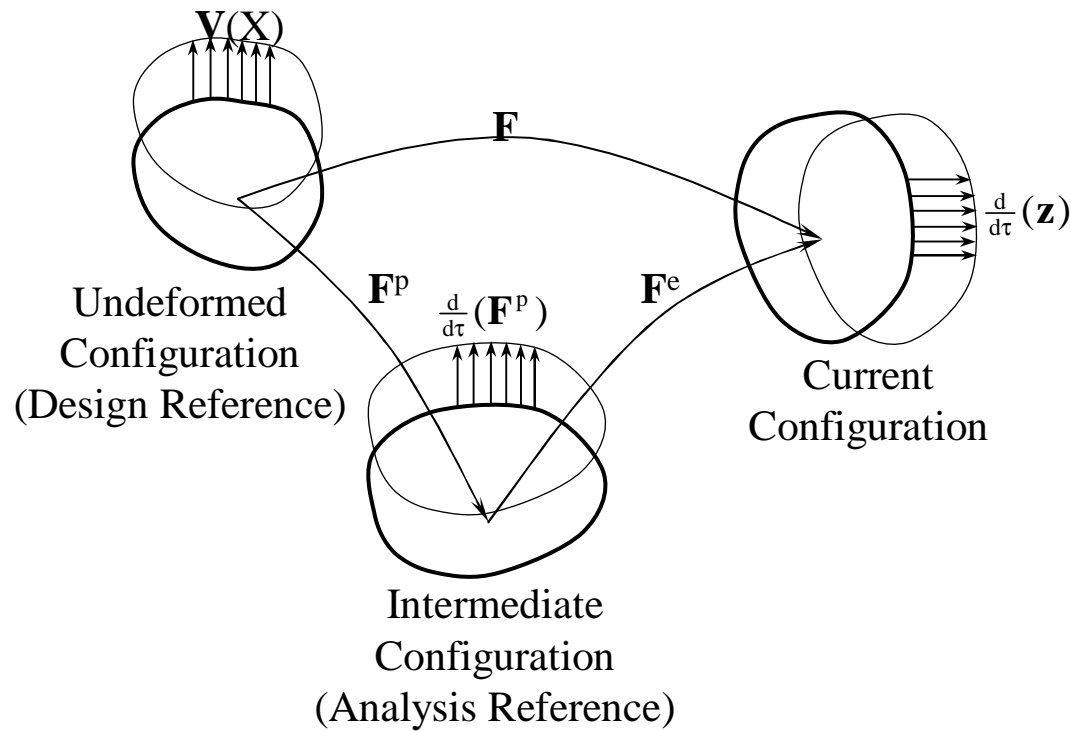
$$\begin{aligned} d(\Delta \mathbf{z}_{,tt}^{k+1}, \bar{\mathbf{z}}) + \beta \Delta t^2 a^*({}^n \mathbf{z}^k; \Delta \mathbf{z}_{,tt}^{k+1}, \bar{\mathbf{z}}) \\ = \ell(\bar{\mathbf{z}}) - a({}^n \mathbf{z}^k, \bar{\mathbf{z}}) - d({}^n \mathbf{z}_{,tt}^k, \bar{\mathbf{z}}), \quad \forall \bar{\mathbf{z}} \in Z \end{aligned}$$

- Update Kinematic Variables

$${}^n \mathbf{z}^{k+1} = {}^n \mathbf{z}^k + \Delta \mathbf{z}^{k+1}$$

$${}^n \mathbf{z}^0 = {}^n \mathbf{z}^{pr}$$

# Finite Deformation DSA



- Updated Lagrangian Formulation
- Finite Deformation Elastoplasticity
- No Need to Update Velocity Fields
- Updating Sensitivity Information of Intermediate Configuration and Plastic Variables

# Finite Deformation DSA *cont.*

## ➤ Material Derivative

$$\dot{\mathbf{z}} = \frac{d}{d\tau}(\mathbf{z})$$

$\mathbf{V}(\mathbf{X})$  : Design Velocity Field

$$= \lim_{\tau \rightarrow 0} \frac{1}{\tau} [\mathbf{z}_{\tau}(\mathbf{X} + \tau \mathbf{V}) - \mathbf{z}(\mathbf{X})]$$

$$= \mathbf{z}' + \nabla_0 \mathbf{z} \mathbf{V}$$

## ➤ Material Derivative of Structural Energy Form

$$\frac{d}{d\tau} [a(\mathbf{z}, \bar{\mathbf{z}})] = a^* (\mathbf{z}, \dot{\mathbf{z}}, \bar{\mathbf{z}}) + a'_V (\mathbf{z}, \bar{\mathbf{z}})$$

Explicitly Dependent Terms on  $\mathbf{V}(\mathbf{X})$   
Path-Dependent Terms

Implicitly Dependent Terms  
Same As Structural Linearization

# Finite Deformation DSA *cont.*

## ➤ Fictitious load

$$a'_V(\mathbf{z}, \bar{\mathbf{z}}) = \int_{\Omega} \left( \varepsilon_{ij}(\bar{\mathbf{z}}) c_{ijkl} \varepsilon_{kl}^P(\mathbf{z}) + \tau_{ij} \eta_{ij}^P(\mathbf{z}, \bar{\mathbf{z}}) + \tau_{ij}^{fic} \varepsilon_{ij}(\bar{\mathbf{z}}) \right) d\Omega \\ + \int_{\Omega} \left( \varepsilon_{ij}(\bar{\mathbf{z}}) c_{ijkl} \varepsilon_{kl}^V(\mathbf{z}) + \tau_{ij} \eta_{ij}^V(\mathbf{z}, \bar{\mathbf{z}}) + \tau_{ij} \varepsilon_{ij}(\bar{\mathbf{z}}) \operatorname{div} \mathbf{V} \right) d\Omega$$

### • Explicitly Dependent Terms

$$\varepsilon^V(\mathbf{z}) = -\operatorname{sym}(\nabla_0 \mathbf{z} \nabla_n \mathbf{V})$$

$$\eta^V(\mathbf{z}, \bar{\mathbf{z}}) = -\operatorname{sym}(\nabla_n \bar{\mathbf{z}}^T \nabla_0 \mathbf{z} \nabla_n \mathbf{V}) \\ -\operatorname{sym}(\nabla_0 \bar{\mathbf{z}} \nabla_n \mathbf{V})$$

### • Path-Dependent Terms

$$\varepsilon^P(\mathbf{z}) = -\operatorname{sym}(\mathbf{G})$$

$$\eta^P(\mathbf{z}, \bar{\mathbf{z}}) = -\operatorname{sym}(\nabla_n \bar{\mathbf{z}}^T \mathbf{G})$$

$$\mathbf{G} = \mathbf{F}^e \frac{d}{d\tau} (\mathbf{F}^P) \mathbf{F}^{-1}$$

$$\tau^{fic} = \sum_{i=1}^3 \left[ \frac{\partial \tau_i^P}{\partial \alpha} \frac{d}{d\tau} (\alpha_n) + \frac{\partial \tau_i^P}{\partial \hat{e}^P} \frac{d}{d\tau} (e_n^P) \right] \mathbf{m}^i$$

# DSA for Structural Dynamics

## ➤ Kinetic Energy

$$\begin{aligned}\frac{d}{d\tau}[d(\mathbf{z}_{,tt}, \bar{\mathbf{z}})] &= \int_{\Omega} \rho \bar{\mathbf{z}}^T \dot{\mathbf{z}}_{,tt} d\Omega + \int_{\Omega} \rho \bar{\mathbf{z}}^T \mathbf{z}_{,tt} \operatorname{div} \mathbf{V} d\Omega \\ &\equiv d(\dot{\mathbf{z}}_{,tt}, \bar{\mathbf{z}}) + d'_V(\mathbf{z}_{,tt}, \bar{\mathbf{z}})\end{aligned}$$

## ➤ Design Sensitivity Equation

$$\frac{d}{d\tau}[d({}^n \mathbf{z}_{,tt}, \bar{\mathbf{z}})] + \frac{d}{d\tau}[a({}^n \mathbf{z}, \bar{\mathbf{z}})] = \frac{d}{d\tau}[\ell(\bar{\mathbf{z}})], \quad \forall \bar{\mathbf{z}} \in Z$$

$$\begin{aligned}d({}^n \dot{\mathbf{z}}_{,tt}, \bar{\mathbf{z}}) + a^*({}^n \mathbf{z}, {}^n \dot{\mathbf{z}}, \bar{\mathbf{z}}) \\ = \ell'_V(\bar{\mathbf{z}}) - a'_V({}^n \mathbf{z}, \bar{\mathbf{z}}) - d'_V({}^n \mathbf{z}_{,tt}, \bar{\mathbf{z}}), \quad \forall \bar{\mathbf{z}} \in Z\end{aligned}$$

– Initial Conditions (Homogeneous)

$$\dot{\mathbf{z}}(\mathbf{x}, 0) = \mathbf{0} \quad \mathbf{x} \in \Omega$$

$$\dot{\mathbf{z}}_{,t}(\mathbf{x}, 0) = \mathbf{0} \quad \mathbf{x} \in \Omega$$

# DSA for Structural Dynamics *cont.*

## ➤ Predictor

$${}^n \dot{\mathbf{z}}_{,t}^{pr} = {}^{n-1} \dot{\mathbf{z}}_{,t} + (1 - \gamma) \Delta t \quad {}^{n-1} \dot{\mathbf{z}}_{,tt}$$

$${}^n \mathbf{z}^{pr} = {}^{n-1} \mathbf{z} + \Delta t \quad {}^{n-1} \dot{\mathbf{z}}_{,t} + \left(\frac{1}{2} - \beta\right) \Delta t^2 \quad {}^{n-1} \dot{\mathbf{z}}_{,tt}$$

## ➤ Corrector

$${}^n \dot{\mathbf{z}}_{,t} = {}^n \dot{\mathbf{z}}_{,t}^{pr} + \gamma \Delta t \quad {}^n \dot{\mathbf{z}}_{,tt}$$

$${}^n \mathbf{z} = {}^n \mathbf{z}^{pr} + \beta \Delta t^2 \quad {}^n \dot{\mathbf{z}}_{,tt}$$

## ➤ Acceleration Form DSA

Sensitivity Equation Is Linear  
and Solves for Total Acceleration

$$d({}^n \dot{\mathbf{z}}_{,tt}, \bar{\mathbf{z}}) + \beta \Delta t^2 a^*({}^n \mathbf{z}; {}^n \dot{\mathbf{z}}_{,tt}, \bar{\mathbf{z}})$$

$$= \ell'_v(\bar{\mathbf{z}}) - a'_v({}^n \mathbf{z}, \bar{\mathbf{z}})$$

$$-d'_v({}^n \mathbf{z}_{,tt}, \bar{\mathbf{z}}) - a^*({}^n \mathbf{z}; {}^n \dot{\mathbf{z}}^{pr}, \bar{\mathbf{z}}), \quad \forall \bar{\mathbf{z}} \in Z$$



# Update Path-Dependent Variables

## ➤ Updating Plastic Variables

$$\frac{d}{d\tau}(\boldsymbol{\alpha}_{n+1}) = \frac{d}{d\tau}(\boldsymbol{\alpha}_n) + \left( H_\alpha + \sqrt{\frac{2}{3}} H'_\alpha \gamma \right) \frac{d}{d\tau}(\gamma) \mathbf{N} + H_\alpha \gamma \frac{d}{d\tau}(\mathbf{N})$$

$$\frac{d}{d\tau}(e_{n+1}^p) = \frac{d}{d\tau}(e_n^p) + \sqrt{\frac{2}{3}} \frac{d}{d\tau}(\gamma)$$

$$\frac{d}{d\tau}(\mathbf{N}) = \frac{1}{\|\boldsymbol{\eta}^{tr}\|} \left[ \mathbf{I}_{dev} - \mathbf{N} \otimes \mathbf{N} \right] \left[ 2\mu \frac{d}{d\tau}(\mathbf{e}^{tr}) - \frac{d}{d\tau}(\boldsymbol{\alpha}_n) \right]$$

$$\frac{d}{d\tau}(\gamma) = A \mathbf{N}^T \left[ 2\mu \frac{d}{d\tau}(\mathbf{e}^{tr}) - \frac{d}{d\tau}(\boldsymbol{\alpha}_n) \right] - A \kappa' \frac{d}{d\tau}(e_n^p)$$

## ➤ Updating Intermediate Configuration

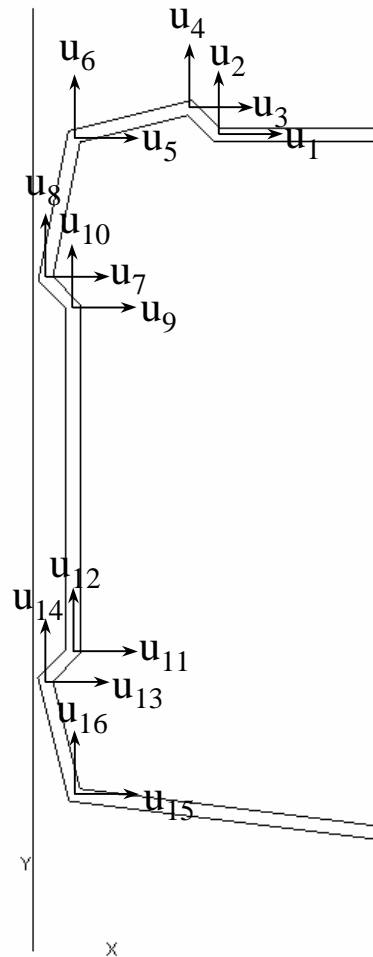
$$\frac{d}{d\tau}(\mathbf{F}_{n+1}) = \frac{d}{d\tau}(\mathbf{I} + \nabla_0 \mathbf{z}) = \nabla_0 \dot{\mathbf{z}} - \nabla_0 \mathbf{z} \nabla_0 \mathbf{V}$$

$$\frac{d}{d\tau}(\mathbf{F}_{n+1}^p) = \frac{d}{d\tau}(\mathbf{F}_{n+1}^{e^{-1}}) \mathbf{F}_{n+1} + \mathbf{F}_{n+1}^{e^{-1}} \frac{d}{d\tau}(\mathbf{F}_{n+1})$$

$$\frac{d}{d\tau}(\mathbf{F}_{n+1}^e) = \frac{d}{d\tau}(\mathbf{f}^p) \mathbf{F}_{n+1}^{e^{tr}} + \mathbf{f}^p \frac{d}{d\tau}(\mathbf{F}_{n+1}^{e^{tr}})$$

$$\frac{d}{d\tau}(\mathbf{F}_{n+1}^{e^{-1}}) = -\mathbf{F}_{n+1}^{e^{-1}} \frac{d}{d\tau}(\mathbf{F}_{n+1}^e) \mathbf{F}_{n+1}^{e^{-1}}$$

# Bumper Impact Problem



default\_Deformation3 :  
Max 2.80-01 @Nd 1

Analysis

Density

Initial Velocity

Analysis Time

Time Increment

Mounting Displ.

Thickness

Contact Penalty No.

Friction Coeff.

Lame's Constants

Plastic Hardening

Initial Yield Stress

Newmark Parameters

Meshfree Method

$\rho = 7,800 \text{ kg/m}^3$

$v_0 = 8.05 \text{ km/hr}$

$t = 0 \sim 10 \text{ msec}$

$\Delta t = 0.1 \text{ msec}$

$d = 2.8 \text{ cm}$

$h = 0.5 \text{ cm}$

$w_n = 1,000$

$\mu_f = 0.4$

$\lambda = 110.8 \text{ GPa}$

$\mu = 80.2 \text{ GPa}$

$H = 1.1 \text{ GPa}$

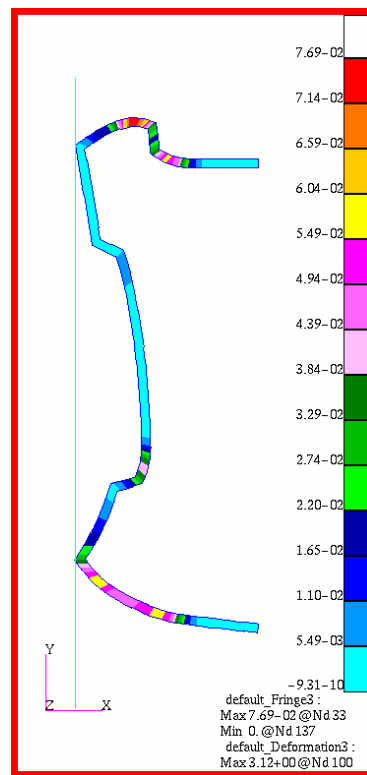
Isotropic Hardening

$\sigma_Y = 500 \text{ MPa}$

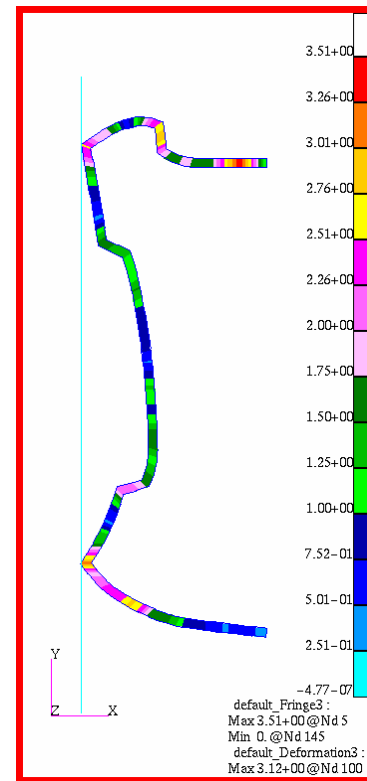
$\gamma = 0.26$

$\beta = 0.5$

# Analysis Results



Effective Plastic Strain

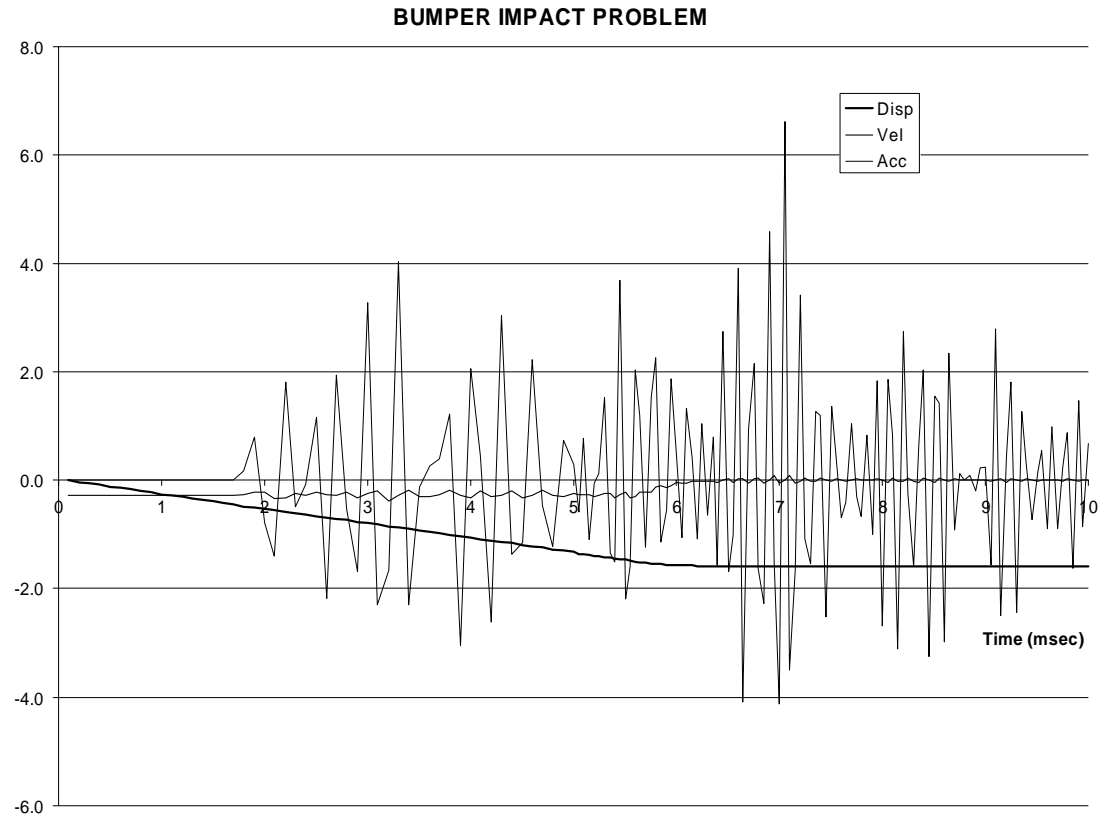


Von Mises Stress

Response Analysis  
1,600 sec

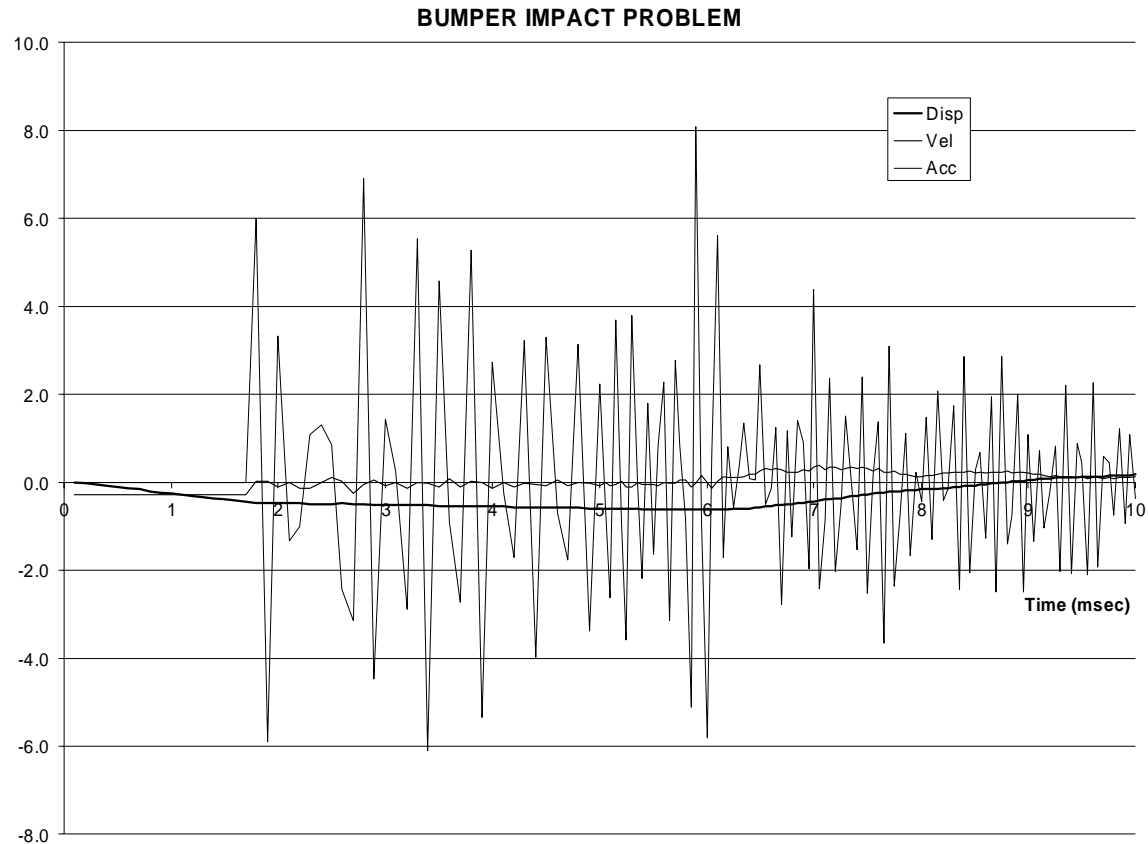
Sensitivity Analysis  
853 / 16 sec

# Time History



Time History of Node 24

# Time History *cont.*



Time History of Node 39

# Sensitivity Results and Optimization Problem

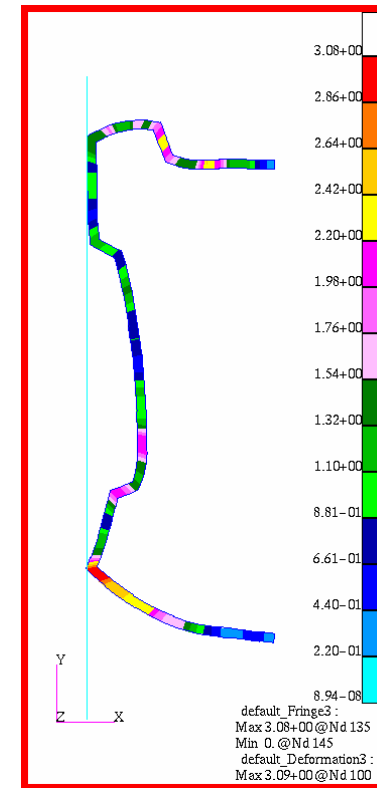
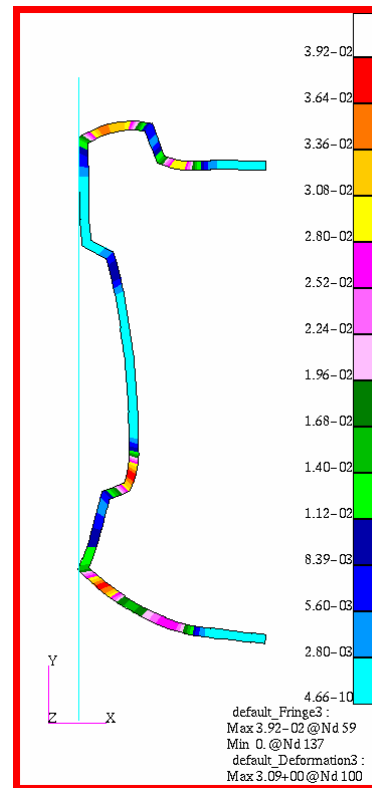
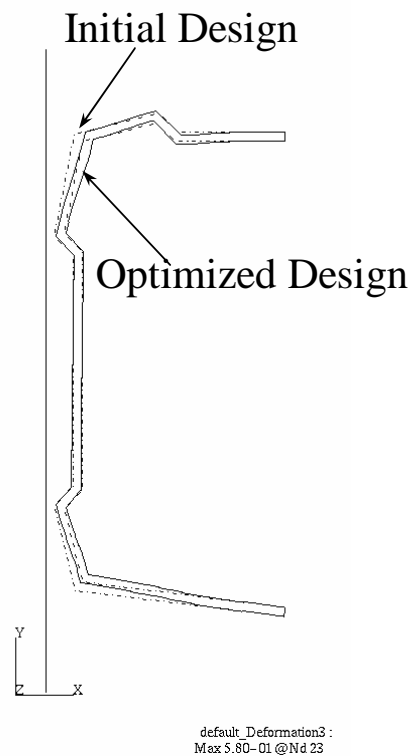
Performance ( $\Psi$ )	$\Delta\Psi$	$\Psi'$	$(\Delta\Psi/\Psi')\times 100\%$	
$u_2$				
$e_{15}^p$	.653533E-01	-.754098E-07	-.754105E-07	100.00
$e_{65}^p$	.618309E-01	.313715E-07	.313668E-07	100.02
$e_{29}^p$	.460146E-01	.441192E-07	.441162E-07	100.01
$Z_{x39}$	.175053E+00	.790973E-05	.791092E-05	99.98
$F_{Cx1.00}$	.128266E+01	-.657499E-06	-.657074E-06	100.06
$u_4$				
$e_{15}^p$	.653533E-01	.268699E-06	.268712E-06	100.00
$e_{65}^p$	.618309E-01	-.843101E-09	-.863924E-09	97.59
$e_{29}^p$	.460146E-01	.123988E-06	.123993E-06	100.00
$Z_{x39}$	.175053E+00	-.847749E-05	-.847586E-05	100.02
$F_{Cx1.00}$	.128266E+01	.410724E-07	.407515E-07	100.79
$u_6$				
$e_{15}^p$	.653533E-01	-.317362E-06	-.317349E-06	100.00
$e_{65}^p$	.618309E-01	-.640031E-07	-.640159E-07	99.98
$e_{29}^p$	.460146E-01	-.163051E-06	-.163051E-06	100.00
$Z_{x39}$	.175053E+00	-.190521E-05	-.190392E-05	100.07
$F_{Cx1.00}$	.128266E+01	.473040E-06	.472876E-06	100.03
$u_8$				
$e_{15}^p$	.653533E-01	.888094E-08	.890589E-08	99.72
$e_{65}^p$	.618309E-01	.355128E-07	.354794E-07	100.09
$e_{29}^p$	.460146E-01	-.981276E-08	-.981572E-08	99.97
$Z_{x39}$	.175053E+00	-.239706E-05	-.239333E-05	100.16
$F_{Cx1.00}$	.128266E+01	-.184457E-06	-.183954E-06	100.27
$u_{10}$				
$e_{15}^p$	.653533E-01	-.642594E-08	-.643542E-08	99.85
$e_{65}^p$	.618309E-01	-.151580E-07	-.151527E-07	100.03
$e_{29}^p$	.460146E-01	.172663E-07	.172698E-07	99.98
$Z_{x39}$	.175053E+00	-.154011E-05	-.154125E-05	99.93
$F_{Cx1.00}$	.128266E+01	-.134372E-06	-.134701E-06	99.76
$u_{12}$				
$e_{15}^p$	.653533E-01	.107017E-07	.107147E-07	99.88
$e_{65}^p$	.618309E-01	-.928369E-07	-.928496E-07	99.99
$e_{29}^p$	.460146E-01	-.163080E-06	-.163083E-06	100.00
$Z_{x39}$	.175053E+00	-.982943E-07	-.957423E-07	102.67
$F_{Cx1.00}$	.128266E+01	-.120596E-05	-.120568E-05	100.02

## Design Optimization Problem Definition

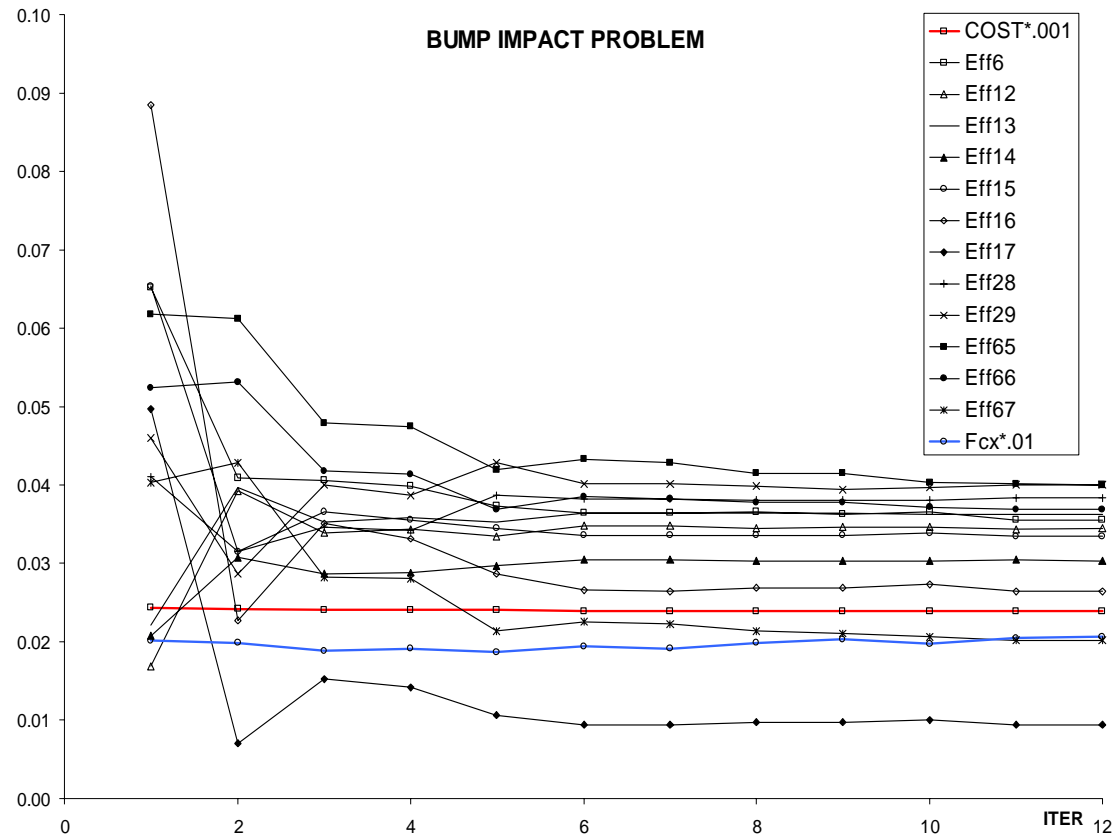
*MIN* Area

*S.T.*  $e_{6}^p(0.07) \leq 0.04$        $e_{7}^p(0.02) \leq 0.04$   
 $e_{12}^p(0.02) \leq 0.04$        $e_{13}^p(0.02) \leq 0.04$   
 $e_{14}^p(0.02) \leq 0.04$        $e_{15}^p(0.07) \leq 0.04$   
 $e_{16}^p(0.09) \leq 0.04$        $e_{17}^p(0.05) \leq 0.04$   
 $e_{28}^p(0.04) \leq 0.04$        $e_{29}^p(0.05) \leq 0.04$   
 $e_{45}^p(0.01) \leq 0.04$        $e_{46}^p(0.01) \leq 0.04$   
 $e_{65}^p(0.06) \leq 0.04$        $e_{66}^p(0.04) \leq 0.04$   
 $e_{67}^p(0.02) \leq 0.04$        $F_{Cx}(2.0) \geq 2.0$   
 $-1.0 \leq u_i \leq 1.0 \quad i=1,16$

# Optimization Results



# Optimization History



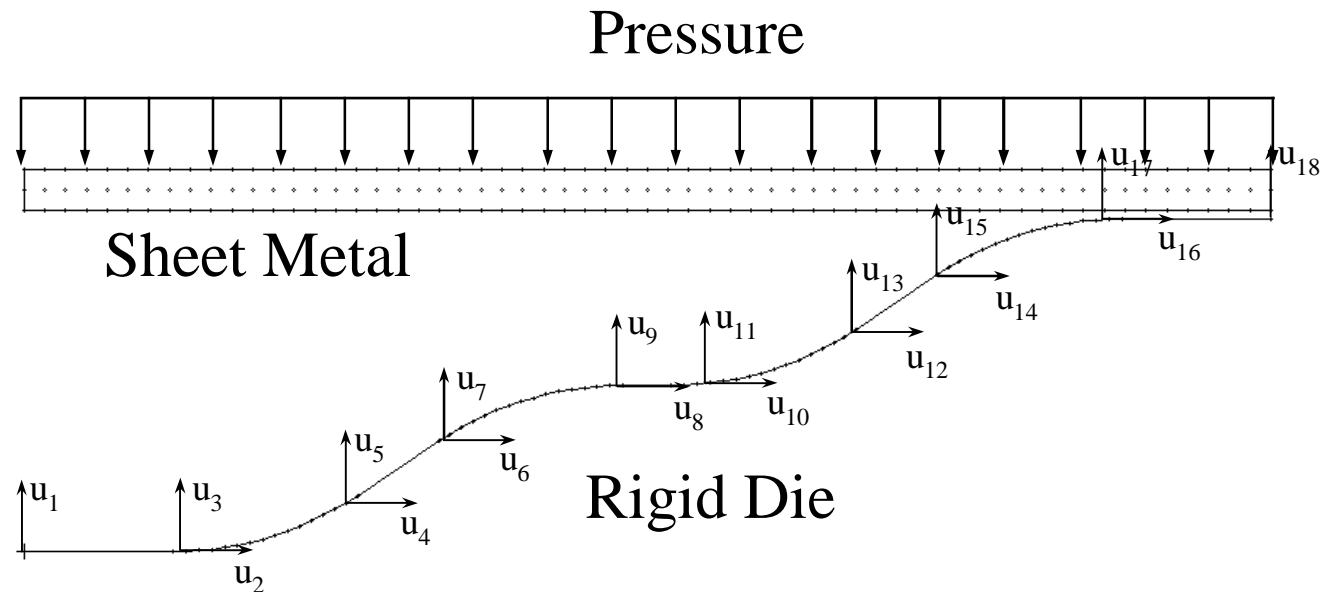
Response Analysis : 30

Sensitivity Analysis : 12



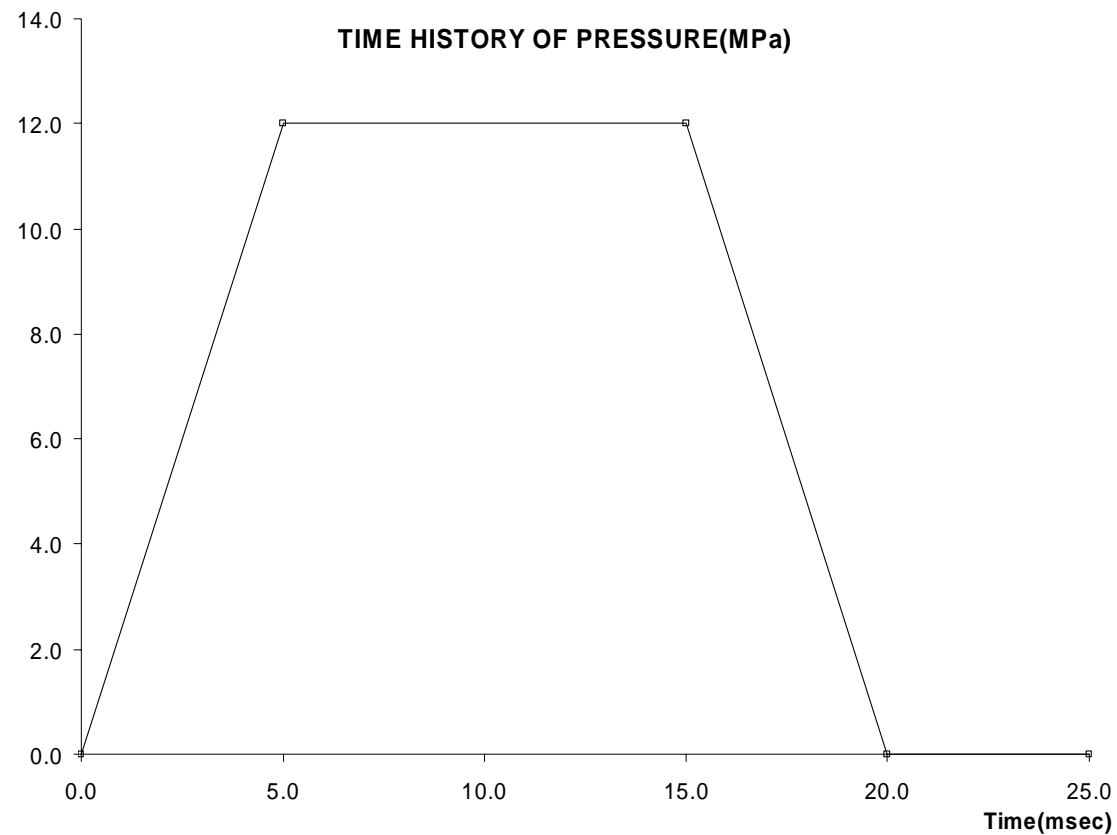
# Pressurized Sheet Metal Stamping

## Initial Geometry and Design Parameters

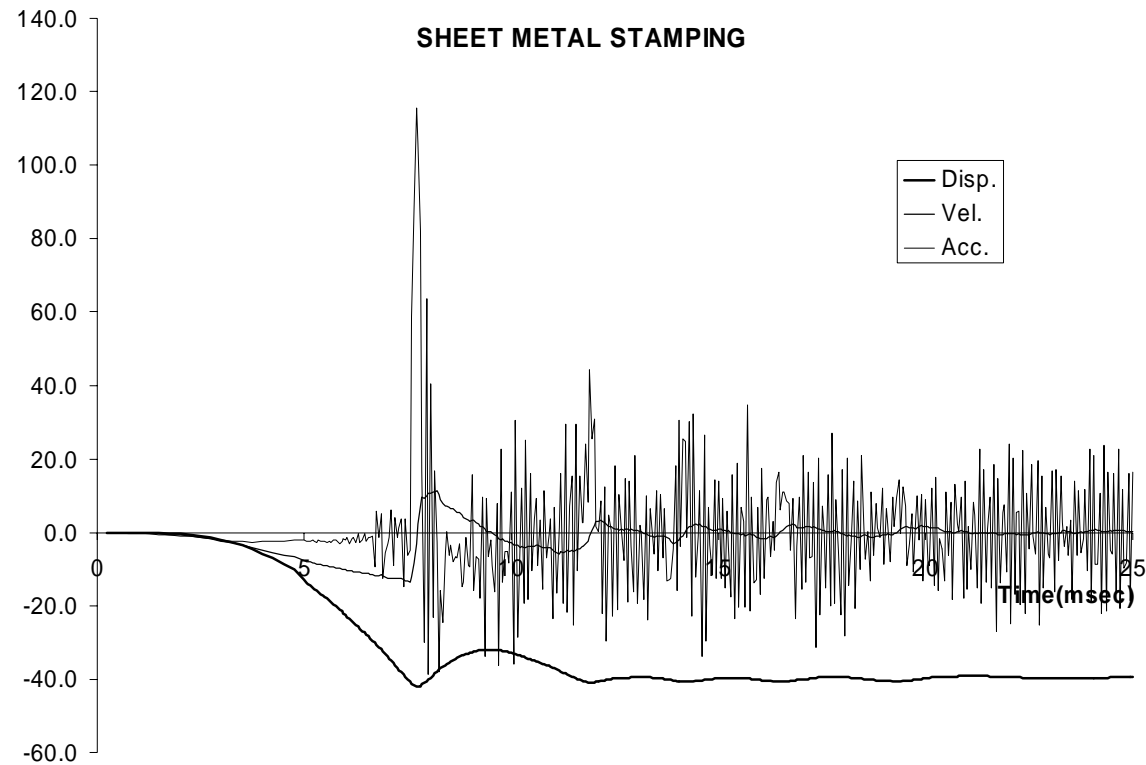


Die Shape DSA and Optimization  
Kim *et al.* *Comp. Mech.* 25 (2000) 157-168

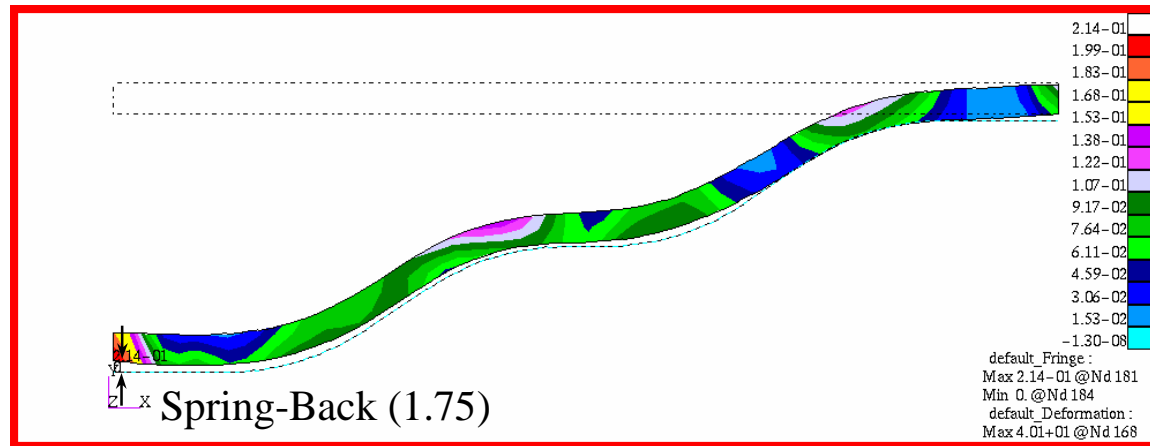
# Pressure Load Time History



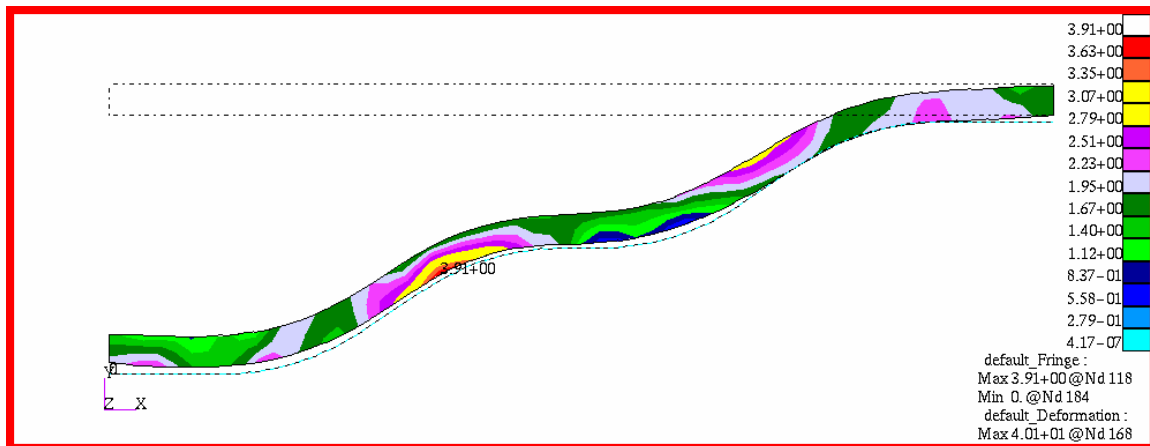
# Time History of Deformation



# Analysis Results



Effective  
Plastic Strain



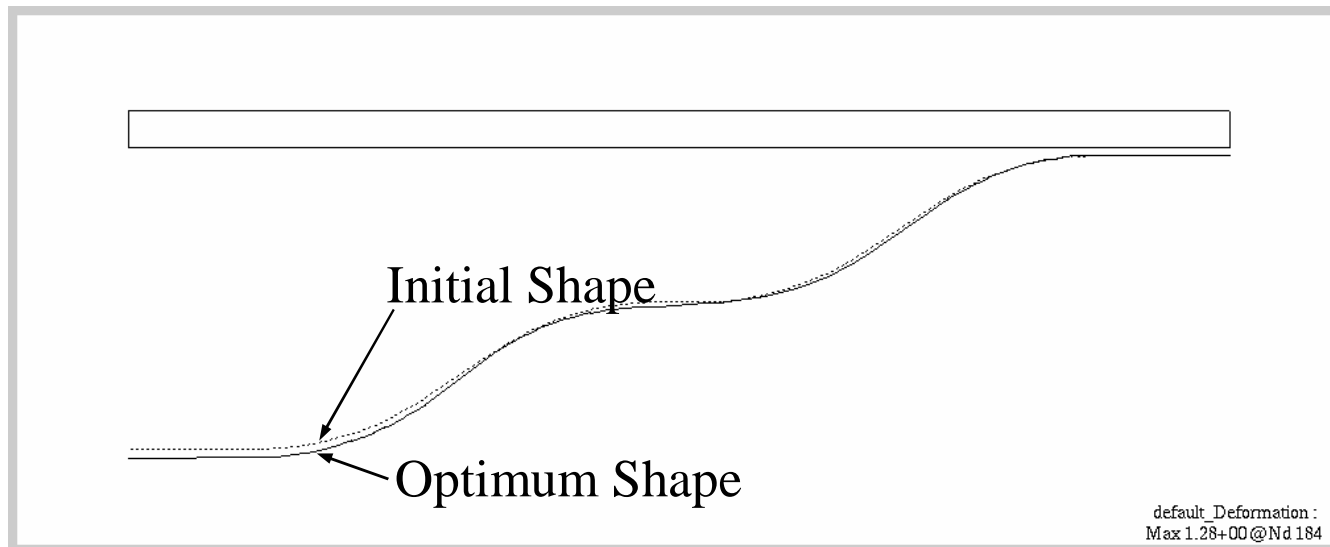
Von Mises  
Stress

# Design Optimization

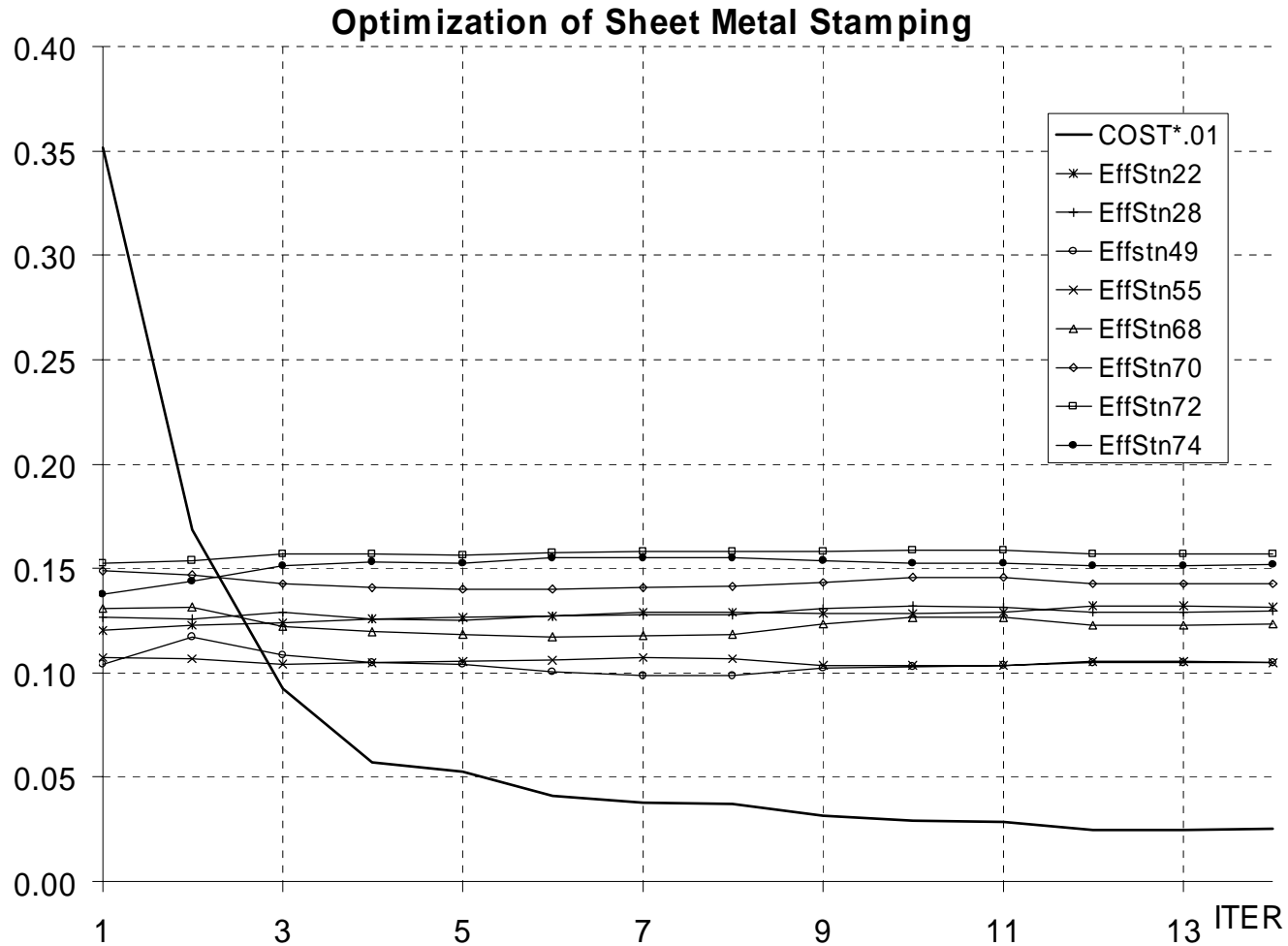
$$\text{MIN} \quad G = \int_{\Gamma} \|\pi(\mathbf{x}) - \mathbf{x}\|^2 \, d\Gamma$$

$$\text{S.T.} \quad e^p_i \leq 0.16 \quad i = 22, 28, 49, 55, \\ 68, 70, 72, 74 \\ -3.0 \leq u_j \leq 3.0 \quad j = 1, \dots, 18$$

Response Analysis : 12,018 sec  
Sensitivity Analysis : 3,215/18 sec



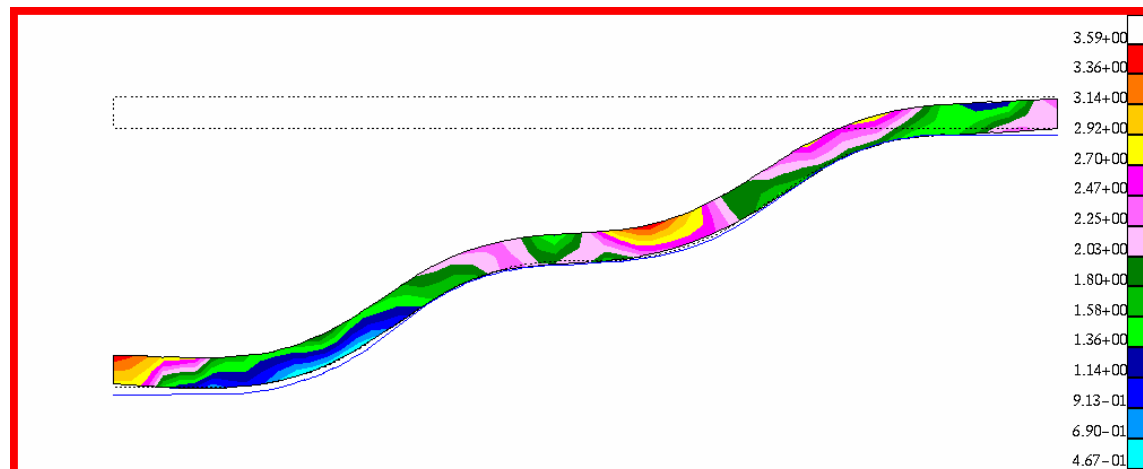
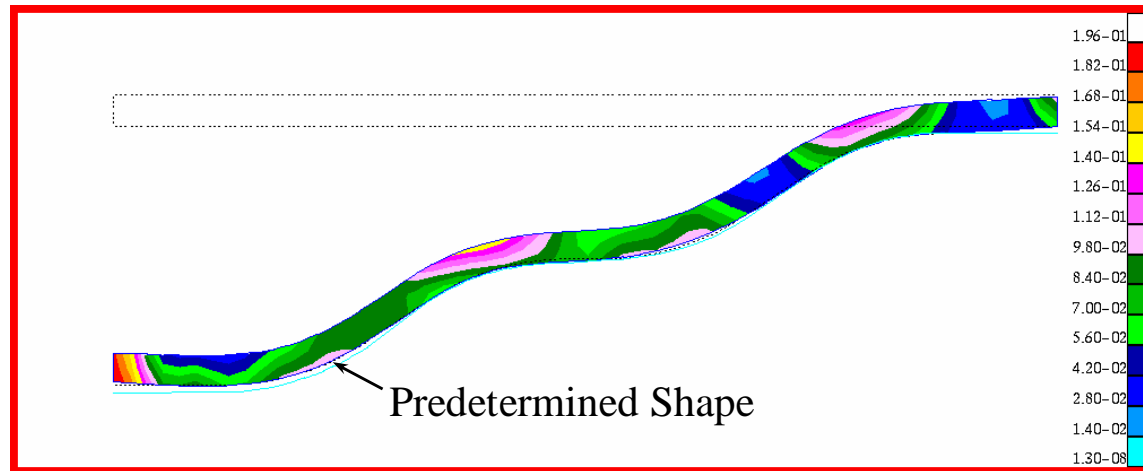
# Optimization History



Response Analysis : 41

Sensitivity Analysis : 14

# Optimization Results



# Conclusions

- An Accurate and Efficient Shape DSA and Optimization of Structural Transient Dynamics is Proposed.
- Finite Deformation Elastoplastic Material and Frictional Contact Condition Are Considered in DSA
- Design Sensitivity Equation Is Solved at Each Converged Time Step without Iteration Using the Same Tangent Stiffness Matrix from Analysis
- Sensitivity Equation Is More Efficient for the Implicit Time Integration Method Than the Explicit Method Compared to the Cost of Response Analysis